

NTNU/XUAT Postgraduate course 21.05.02-31.05.02: Wastewater as a resource

# ADVANCEMENTS IN PHYSICAL/CHEMICAL TREATMENT

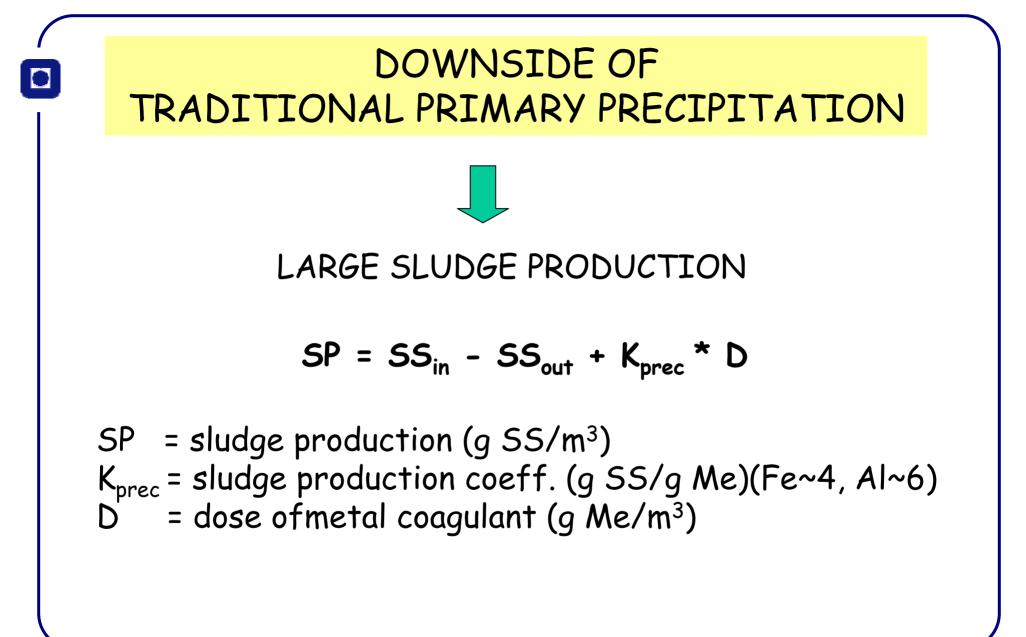
Hallvard Ødegaard

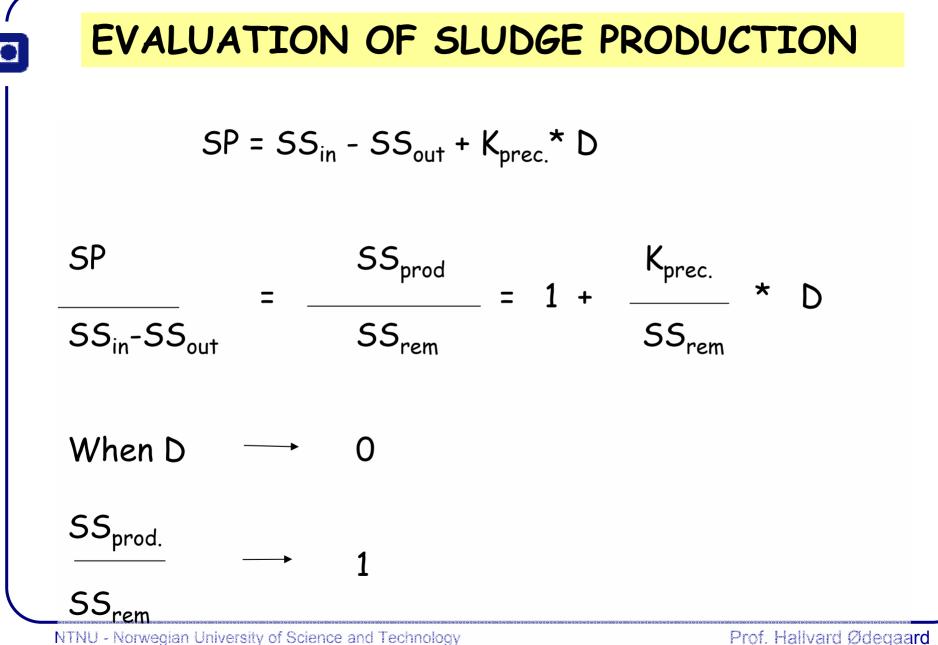
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# CHALLENGES IN THE USE OF COAGULATION OF WASTEWATER

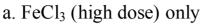
- MINIMISATION OF SLUDGE PRODUCTION
- MINIMISATION OF SPACE REQUIREMENT
  - Flocculation
  - Floc separation
- REMOVAL OF SOLUBLE (ORGANIC) MATTER
- PRODUCTION OF CARBON SOURCE IF NITROGEN REMOVAL IS TO BE ACHIEVED



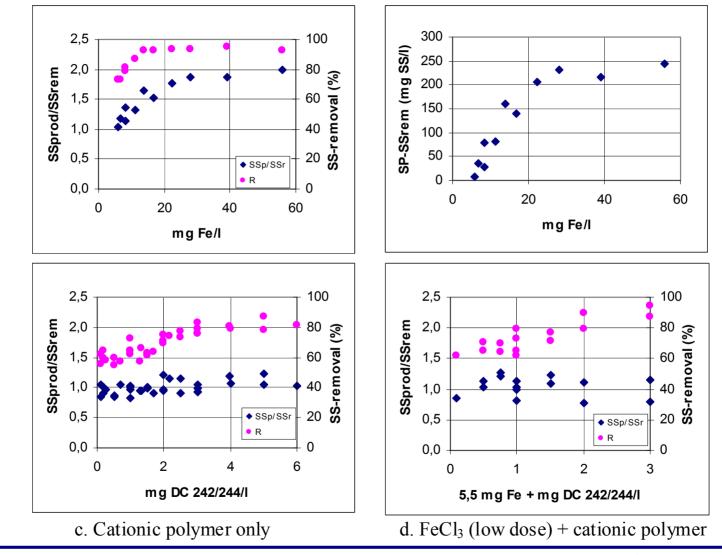


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#### COMPARISON AT DIFFERENT DOSAGE SCENARIOS



b. FeCl<sub>3</sub> (high dose) only



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#### FLOCCULATION - KEY FACTOR

a) Orthokinetic flocculation
\* Turbulent velocity gradient (G)
\* Residence time (T)
\* Residence time distribution (m)

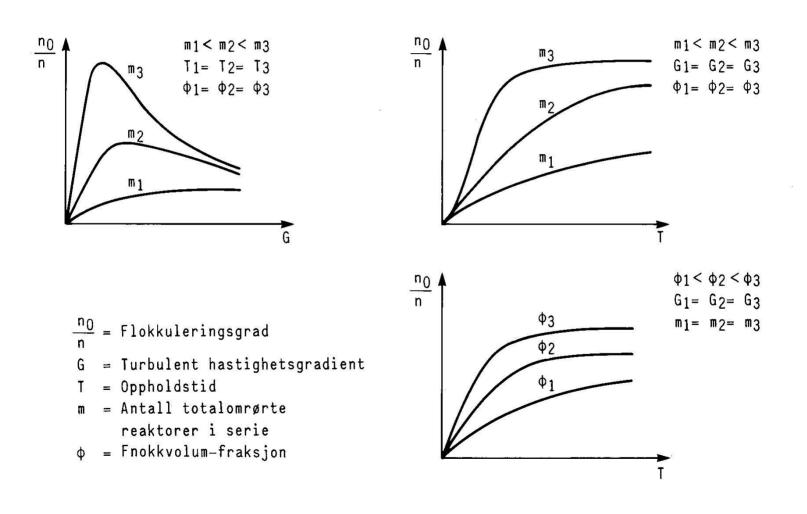
\* Floc volume fraction

 $G = (W/\mu)^{1/2}$ T = V/Q m~number of reactors in series  $\Phi = f(Me, dose)$ 

- b) Chemical flocculation (polymeric flocculants) by anionic polymer addition
  - \* Polymer charge (anionic, cationic, non-ionic)
  - \* Polymer type (polyacrylamide, polyDadmac, polyamin)
  - \* Polymer dose
  - \* Polymer characteristic (MW, Charge density)

# ORTHOKINETIC FLOCCULATION

Relationship between flocculation performance and flocculation variables

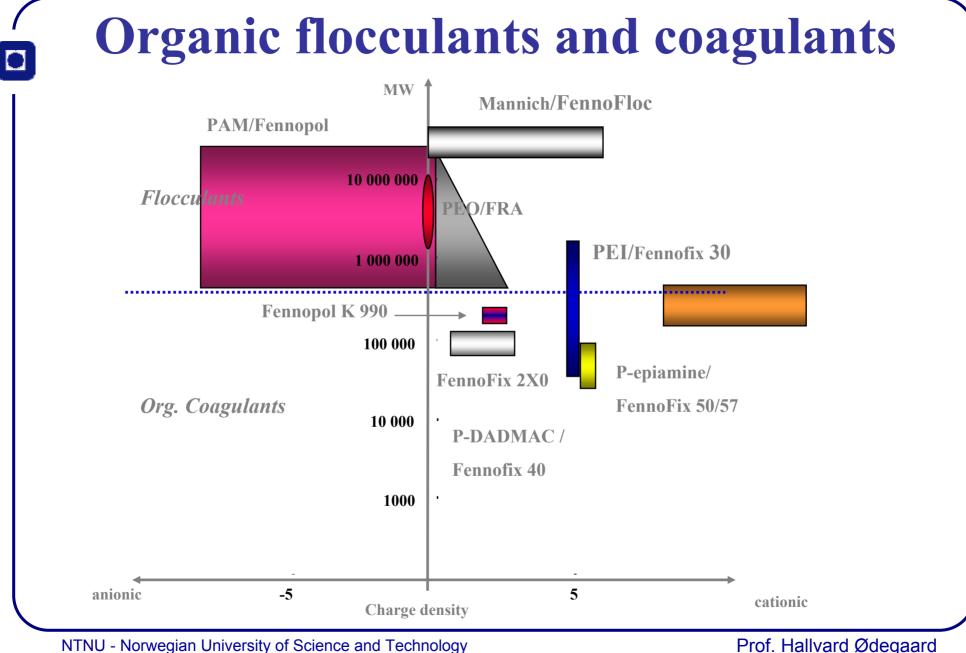




## CHEMICAL FLOCCULATION

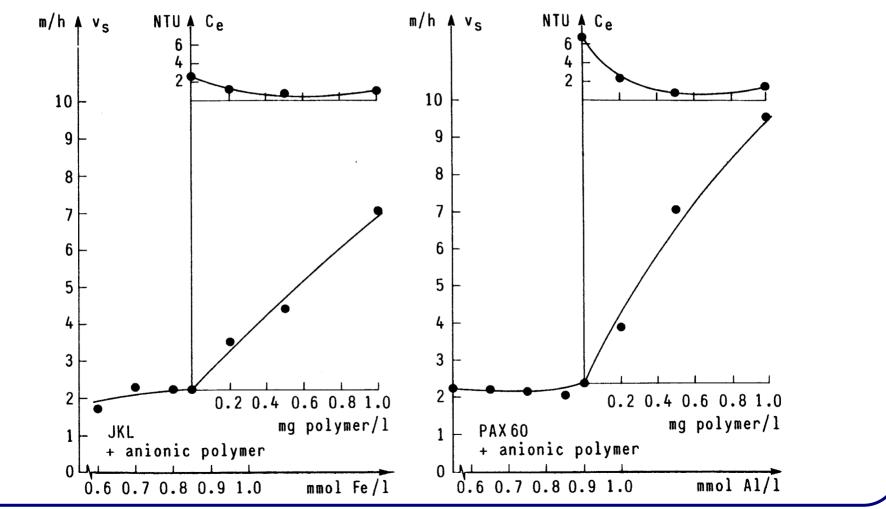
#### INFLUENCE OF ANIONIC POLYMER ADDITION ON CHEMICAL FLOC SETTLING RATE

- 1. Anionic polymer addition leads to larger flocs due to the bridging mechanism
- 2. Low dosage needed (0,1 1,0 mg/l)
- 3. Polymer flocculant addition leads to higher acceptable turbulent velocity gradient and consequently to lower acceptable residence time



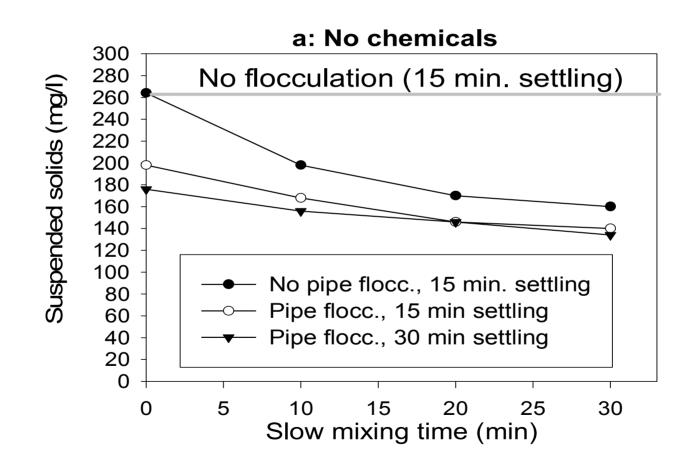
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#### FLOC SETTLING RATE ( $v_s$ ) VERSUS CHEMICAL DOSAGES ( $C_e$ - turbidity of settled effluent)



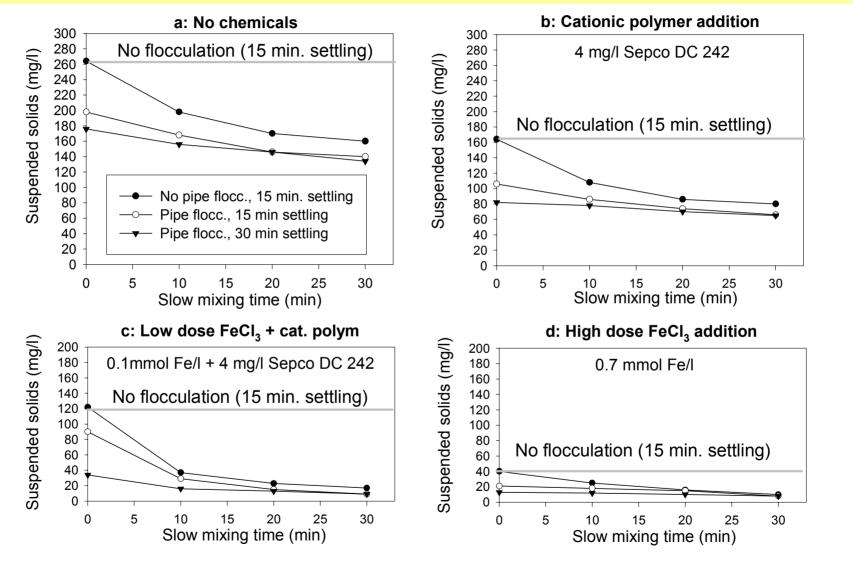
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#### THE IMPORTANCE OF ORTHOKINETIC FLOCCULATION EVEN IN PRIMARY TREATMENT



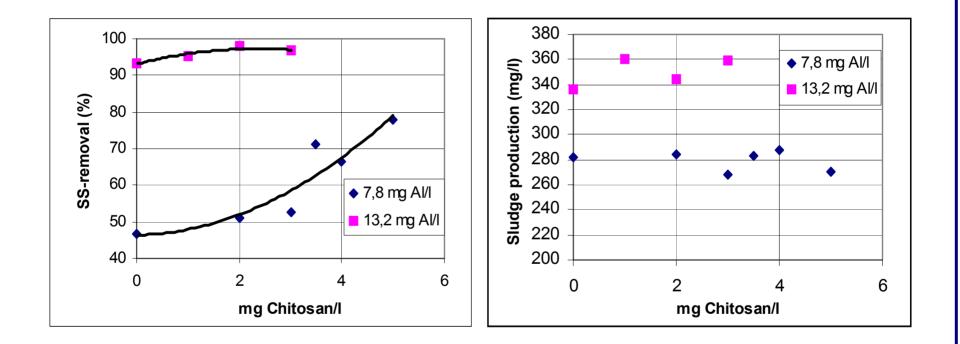
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### COMPARISON AT DIFFERENT DOSAGE SCENARIOS

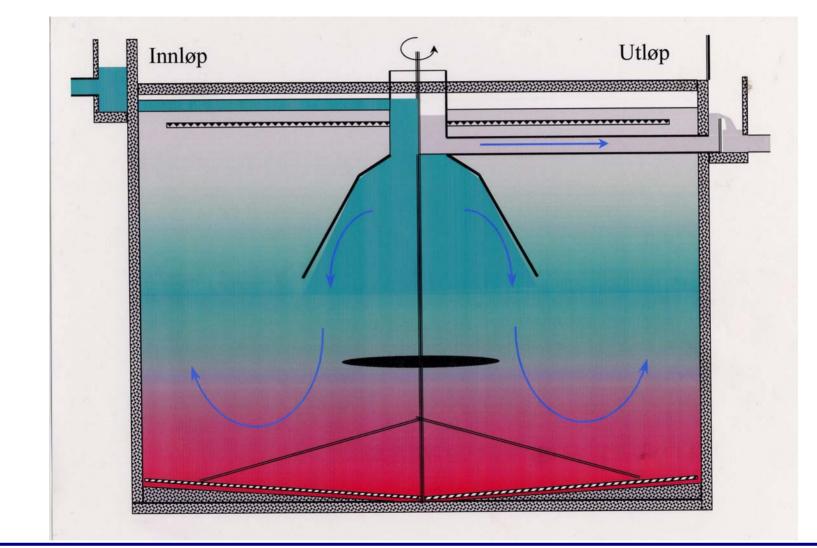


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#### THE BIOPOLYMER CHITOSAN AS REPLACEMENT FOR METAL CATION IN PRIMARY COAGULATION

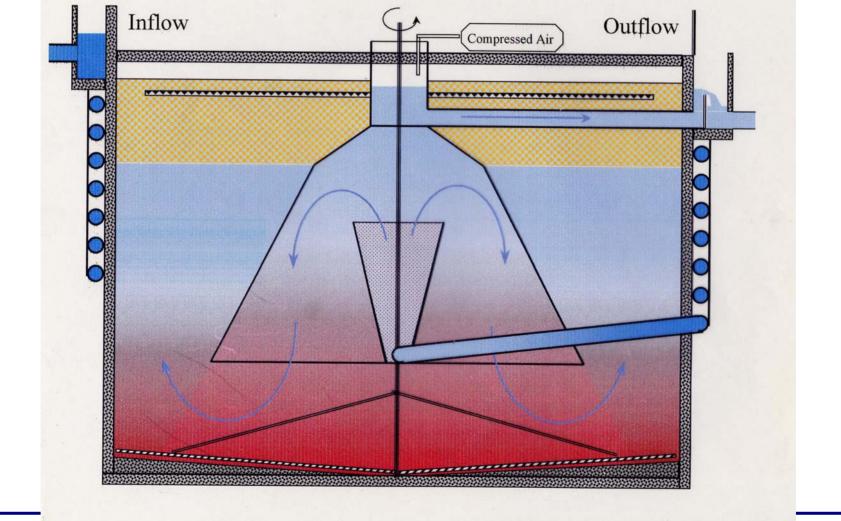


# DEEP SETTLING TANK WITH INTERNAL FLOCCULATION



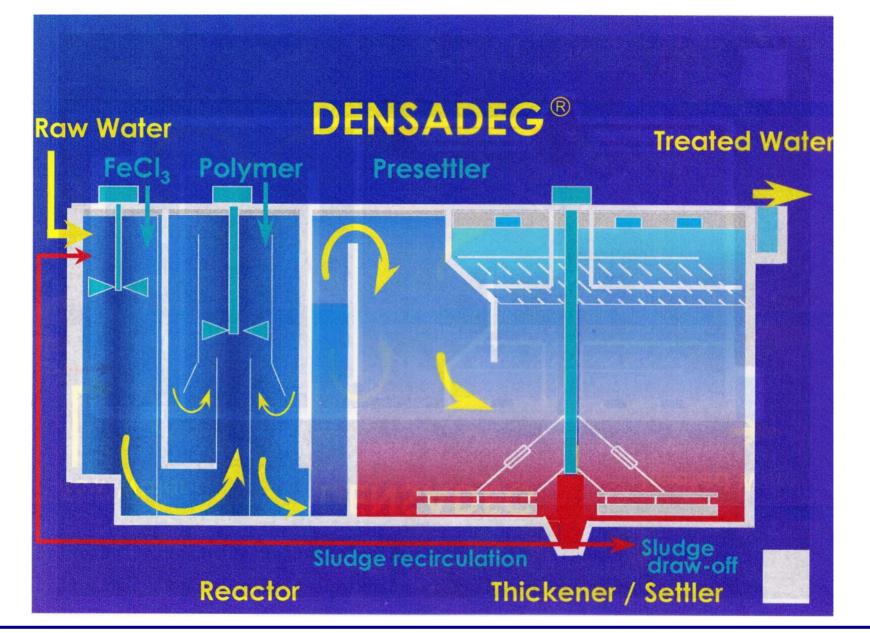
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#### DEEP SETTLING TANK WITH PIPW FLOCCULATION AND COARSE FILTRATION/LAMELLA SEPARATION

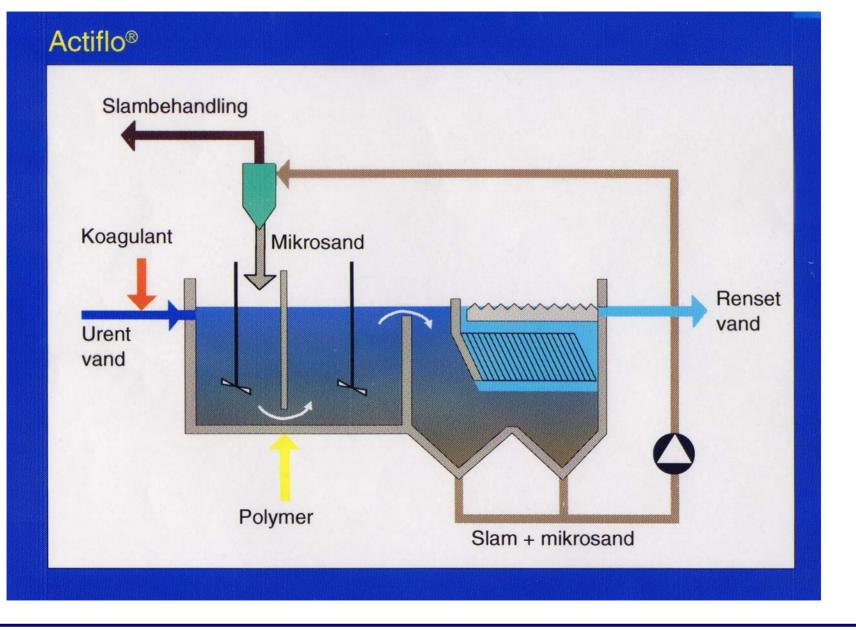


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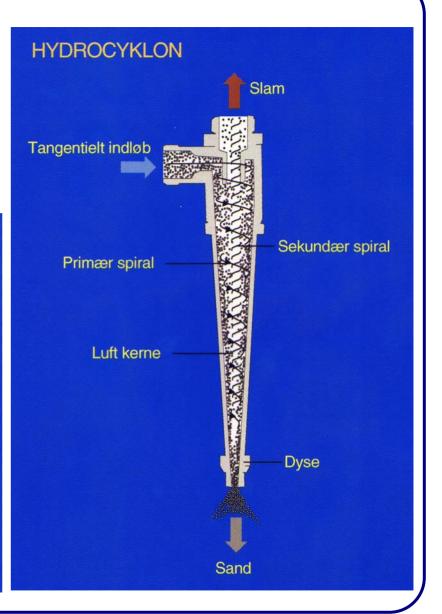


## ACTIFLO MICROSAND SEPARATION

Sandkorn 60 - 180 μm

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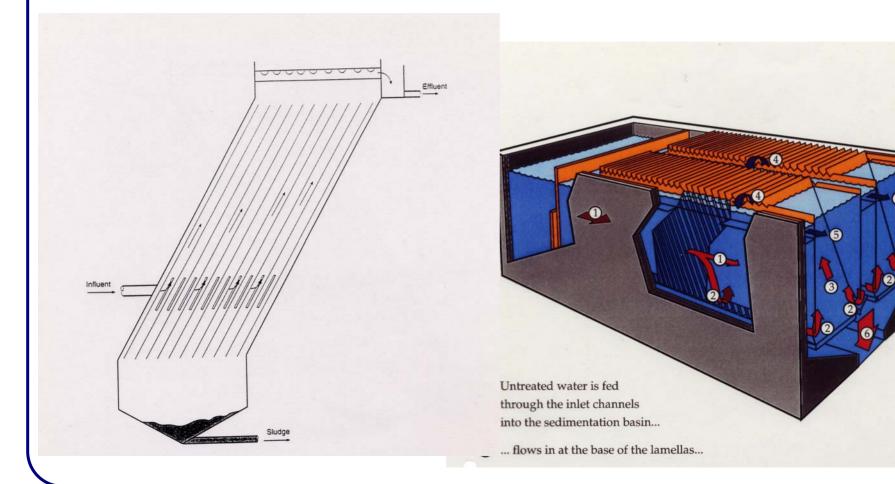




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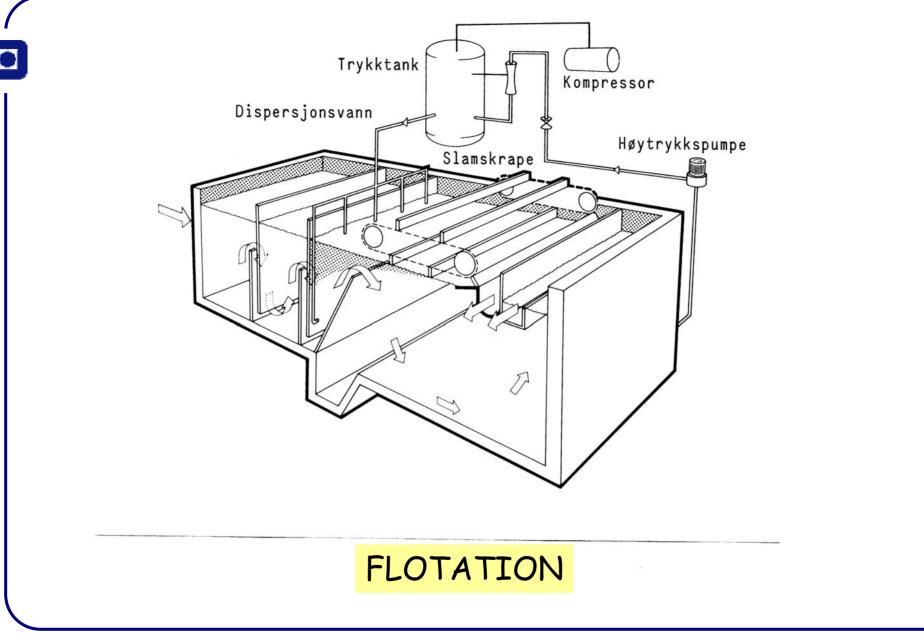


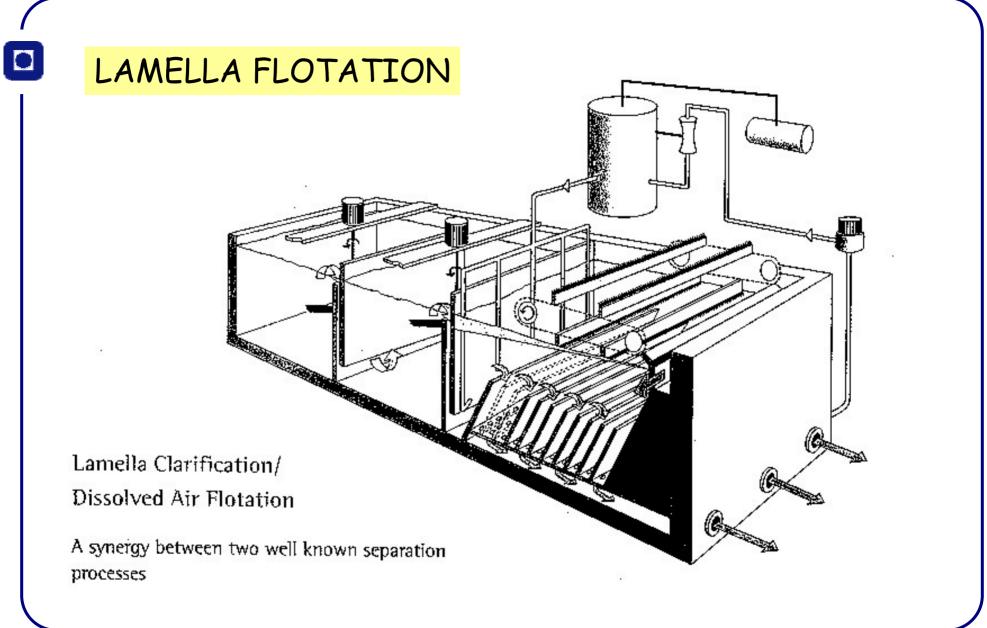
#### LAMELLA SETTLING

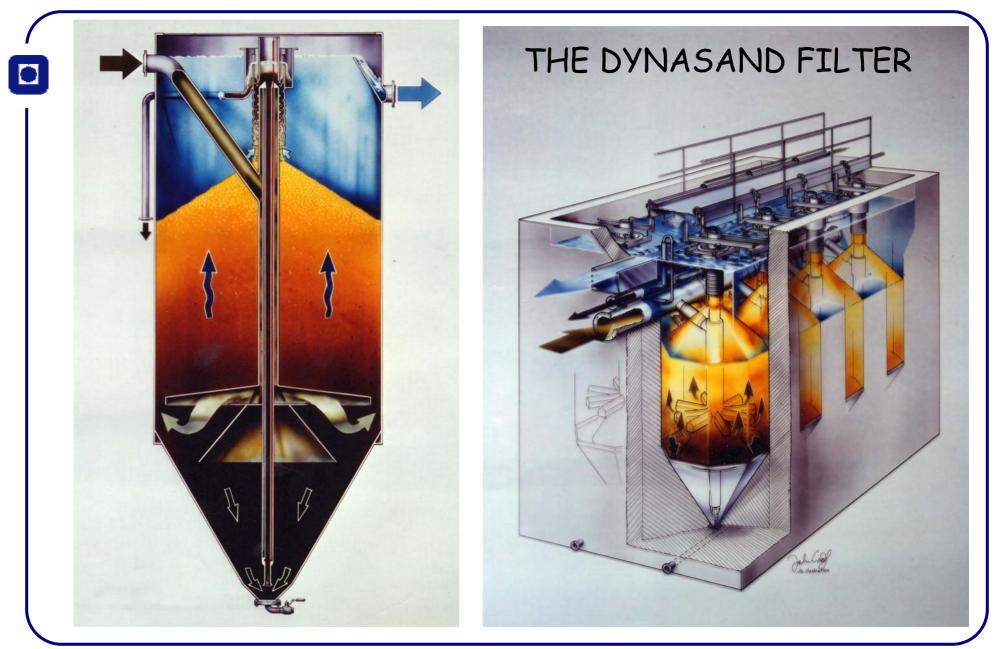


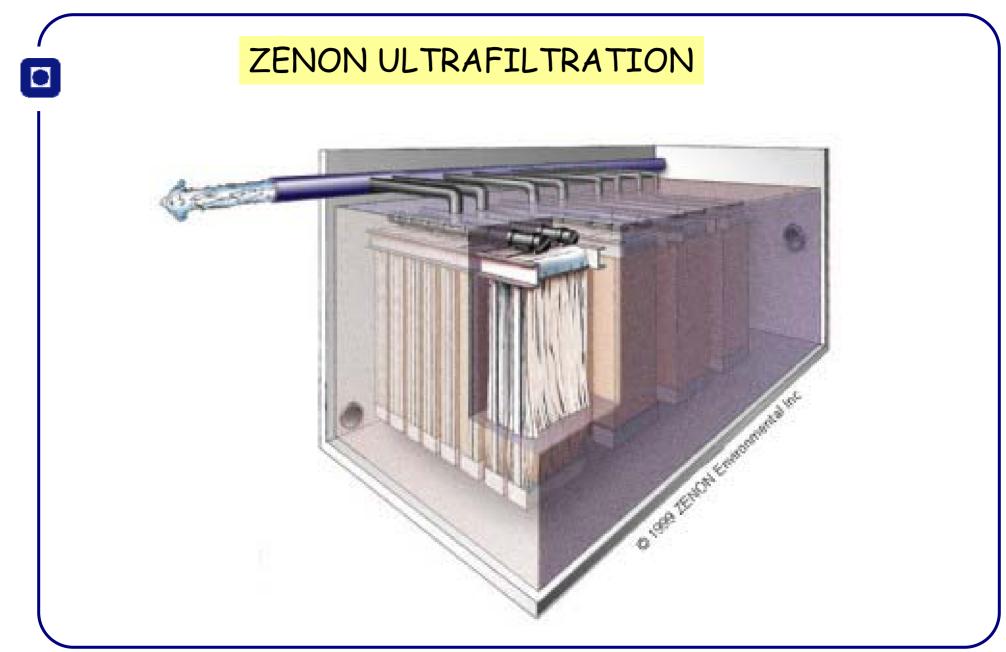
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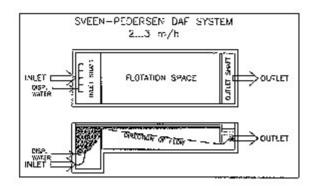


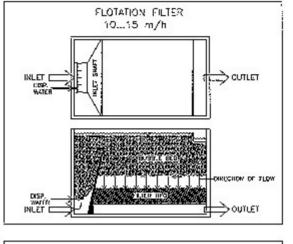


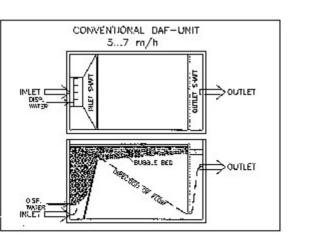


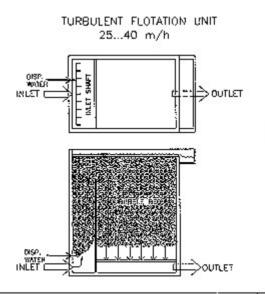


#### DEVELOPMENT OF FLOTATION









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# ADVANTAGES AND DISADVANTAGES OF FLOTATION IN COMPARISON WITH SEDIMENTATION

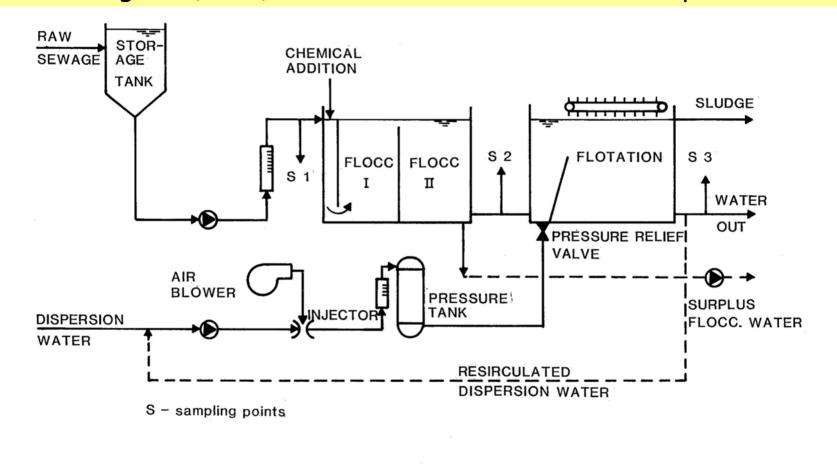
# ADVANTAGES

- Less space required,
   v<sub>f</sub> = 5-15 m/h against 1-2 m/h
- Better separation efficiency
- Higher sludge concentration

# DISADVANTAGES

- Higher costs ?
- Less known technology
- More skilled operators needed ?

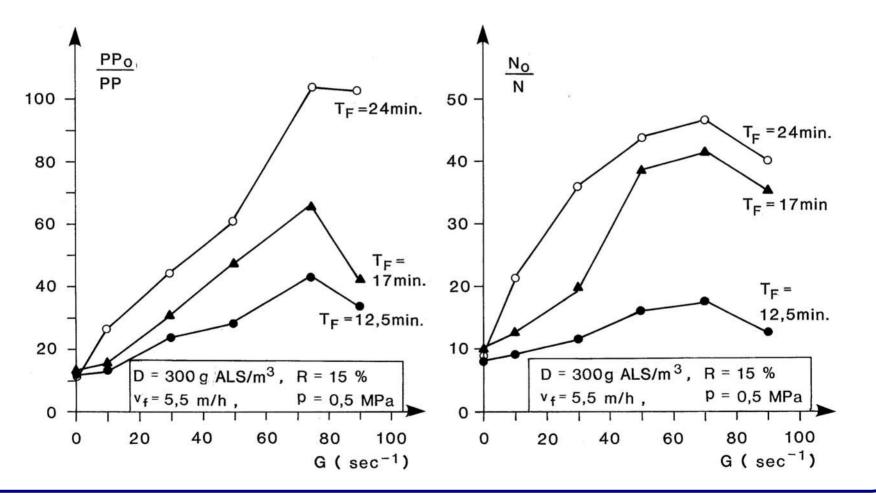
#### FLOCCULATION/FLOTATION EXPERIMENTS Ødegaard(1995) Wat.Sci.Tech. Vol. 31, No 3.-4. Pp 73-82



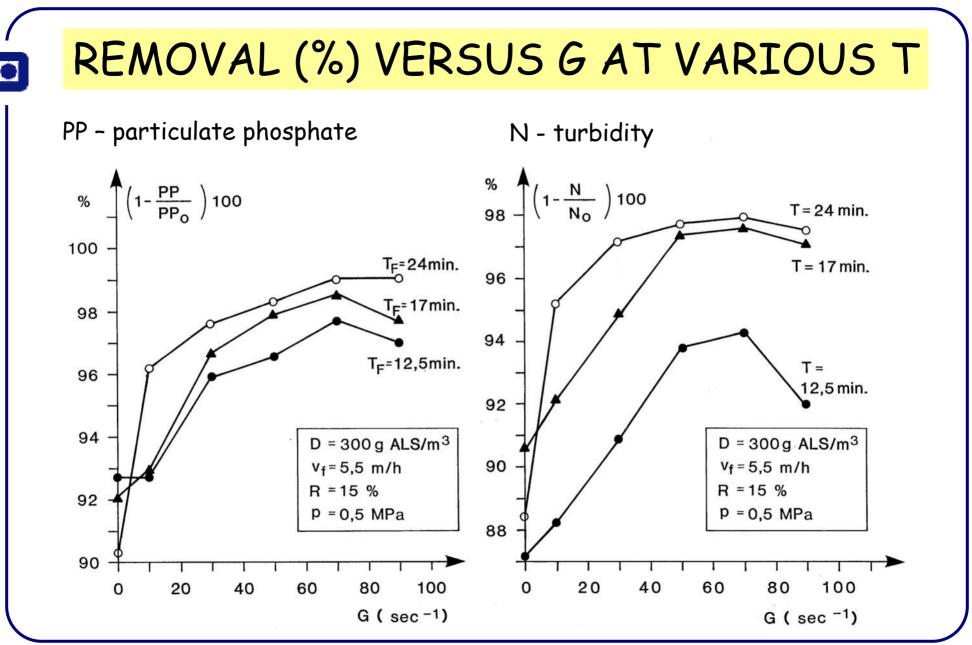
#### FLOW SHEET - EXPERIMENTAL SETUP

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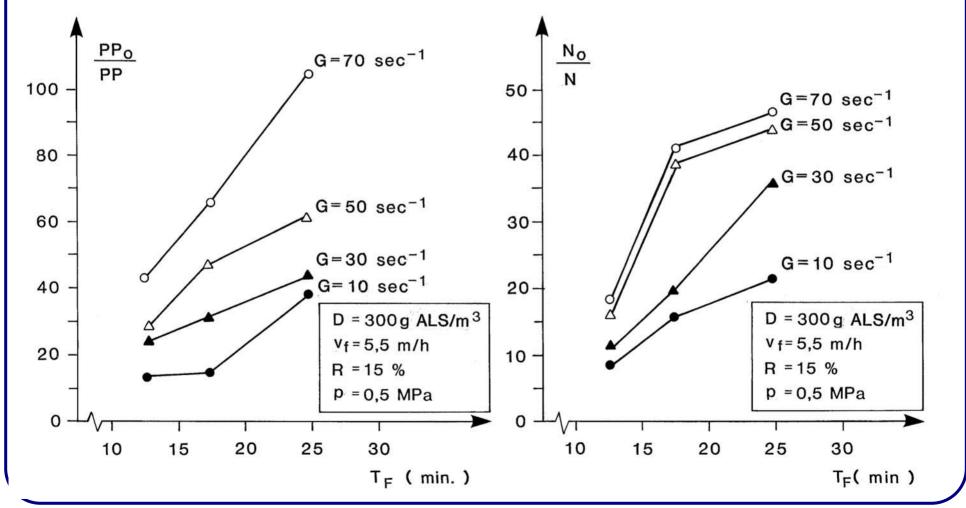
# FLOCCULATION /FLOTATION EFFICIENCY VS G-VALUE AT VARIOUS RESIDENCE TIMES (T)



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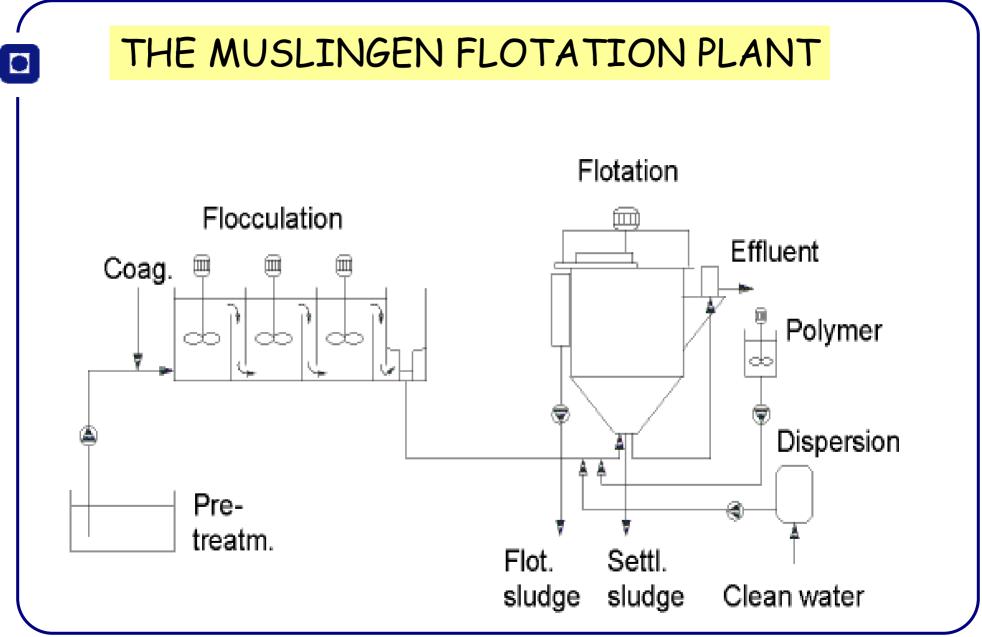
# FLOCCULATION/FLOTATION EFFICIENCY VERSUS T AT VARIOUS G



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## FLOCCULATION AHEAD OF FLOTATION Ødegaard, Wat.Sci.Tech. Vol 31, No3-4, 1995

- 1. The theor.mean residence time at design flow should be 25 30 min.
- 2. The flocculator should be designed to give a residence time distribution as plug flow like as possible. If stirred tanks are used, the flocculator should be divided into at least two chambers
- 3. The G-value should be the same in each of the flocculator chambers and in the order of 60 80 sec<sup>-1</sup>.
- 4. The flotation unit should be designed for a hydraulic surface load of 5 - 6 m<sup>3</sup>/m<sup>2</sup>·h at design flow allowing for variations up to 10 m<sup>3</sup>/m<sup>2</sup>·h at maximum design flow. If the variation in the flow is small, a load of 8 m<sup>3</sup>/m<sup>2</sup>·h could be recommended at design flow.
- 5. The amount of pressurized water should be 10-20 % of design flow when the pressure is 0.5 MPa.





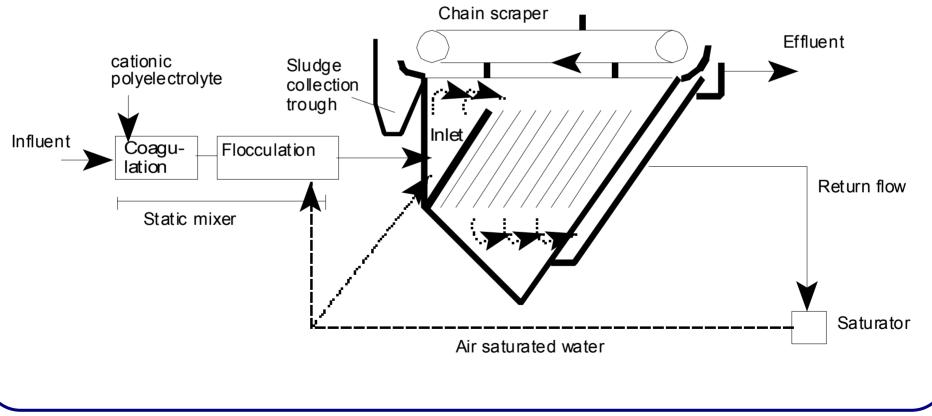
# TREATMENT RESULTS MUSLINGEN

Plant	Overflow	Tot P	Tot P	Tot P
	rate (m/h)	In (mg/l)	Out (mg/l)	%
A	1.9 (4,0)	5,9 (9,3)	0.12 (0,42)	98.0
В	4.2 (8.7)	4,6 (7,3)	0.21 (0.59)	95.4
С	1.9 (6.9)	4.4 (7.6)	0.10 (0.34)	97.7
D	4.6 (7.3)	4.6 (7.3)	0.38 (1.63)	91.7
E	4.4 (6.6)	1.9 (2,7)	0.06 (0.07)	96.8
Plant	Overflow	COD (TOC)	COD (TOC)	COD (TOC)
	rate (m/h)	In (mg/l)	Out (mg/l)	%
A	rate (m/h) 1.9 (4,0)	In (mg/l) 337 (710)	Out (mg/l) 62 (120)	% 81.6
A B				
	1.9 (4,0)	337 (710)	62 (120)	81.6
В	1.9 (4,0) 4.2 (8.7)	337 (710) <i>109 (190)</i>	62 (120) <i>8.9 (15.3)</i>	81.6 <i>91.8</i>

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# Dutch experiments (Mels et al, 2000)

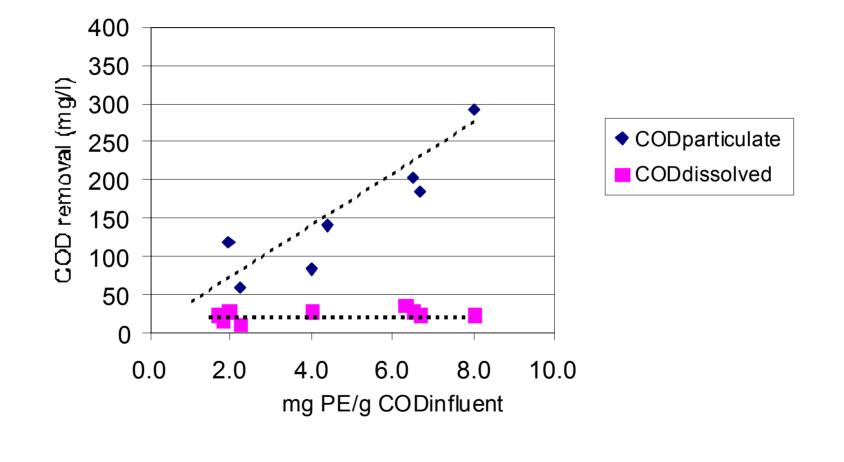
# Pilot system



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# DAF Pilot results: COD removal



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# Discussion: Determining flocculation/flotation efficiency with turbidity measurements

100

80

60

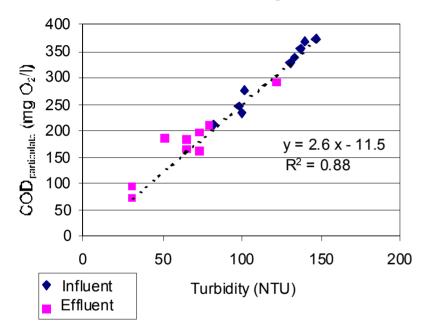
40

20

0

0.0

20D<sub>1 adicuteth</sub> removal(%)



# Linear relation turbidity and particulate COD

Evaluation of two PE's by plotting particulate COD as a function of PE-NTU<sub>influent</sub>-ratio

Setpoint

10.0

mg pe/100 NTU

15.0

 $\checkmark$ 

◆ 8 Mg/mol ▲ 4 Mg/mol

5.0

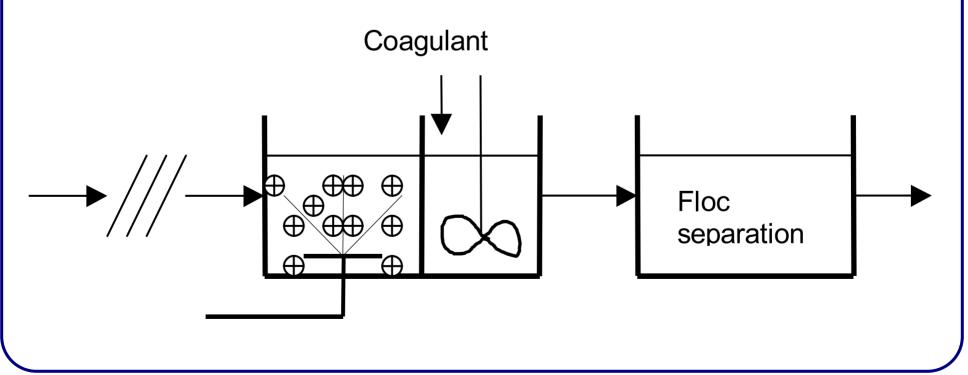
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0

20.0

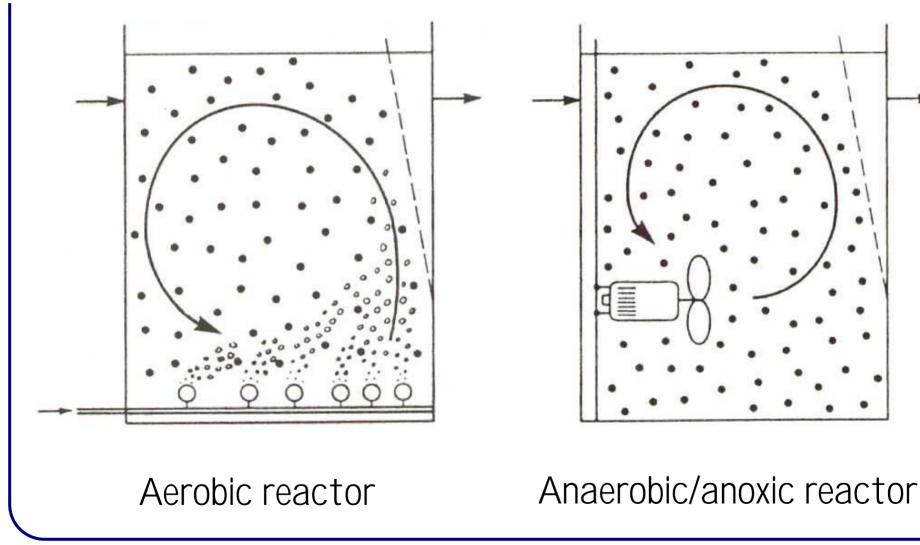
### PROPOSED SECONDARY TREATMENT PROCESS

- Particulate organic matter is coagulated and separated as in a conventional chemical plant
- Easily biodegradable matter is removed in a highly loaded biofilm reactor at such a high load that only easily biodegradable organic matter is degraded (no hydrolysis)
- Good biomass separation of incoming particles as well as produced biomass



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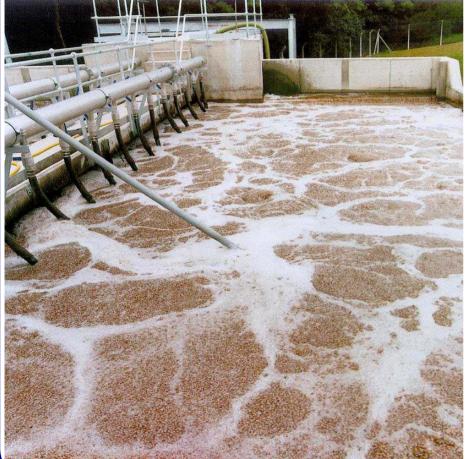
### THE PRINCIPLE OF THE MOVING BED REACTOR



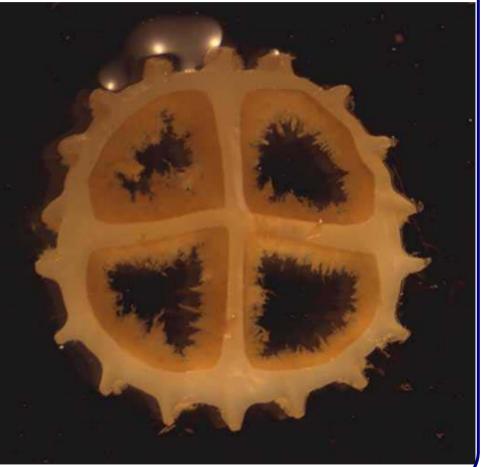
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## THE KALDNES MOVING BED BIOFILM PROCESS

#### Aerated tank for nitrification



Carrier under water



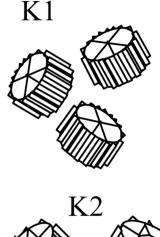
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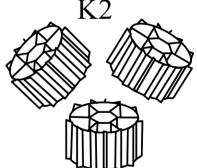
# CHARACTERISTICS OF THE BIOFILM CARRIERS

Biofilm carrier :

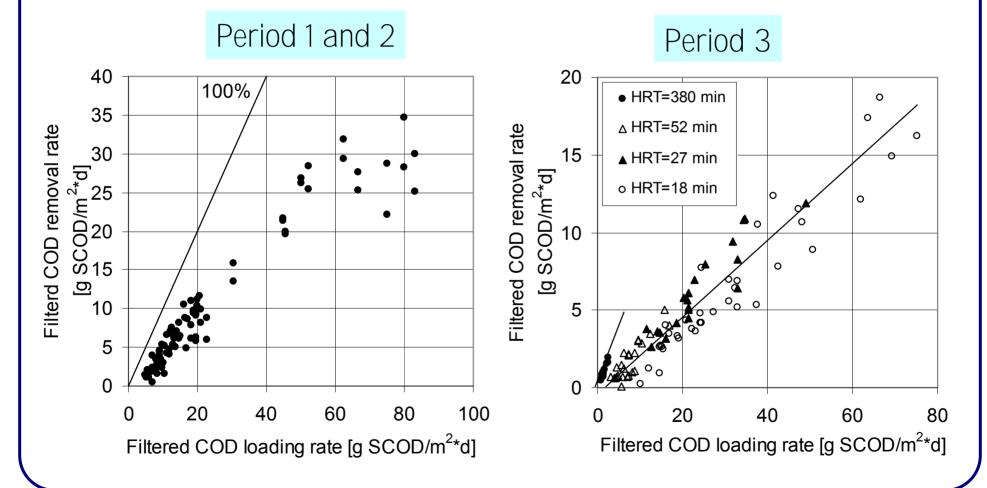
Material : Polyethylene (density 0,95 g/cm<sup>3</sup>) Size : K1 - Diam./Length = 10mm/7mm K2 - Diam./Length = 15mm/15mm

Surface area	K1	K2
Per carrier (mm <sup>2</sup> )		
Total	670	2300
Effective for biofilm growth	490	1530
Specific area $(m^2/m^3)$		
Total at 70 % carrier filling	490	330
Effective for biofilm growth	350	220

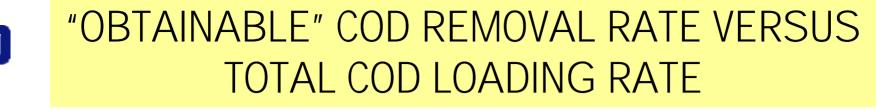




## SOLUBLE COD REMOVAL RATE VERSUS SOLUBLE COD LOADING RATE

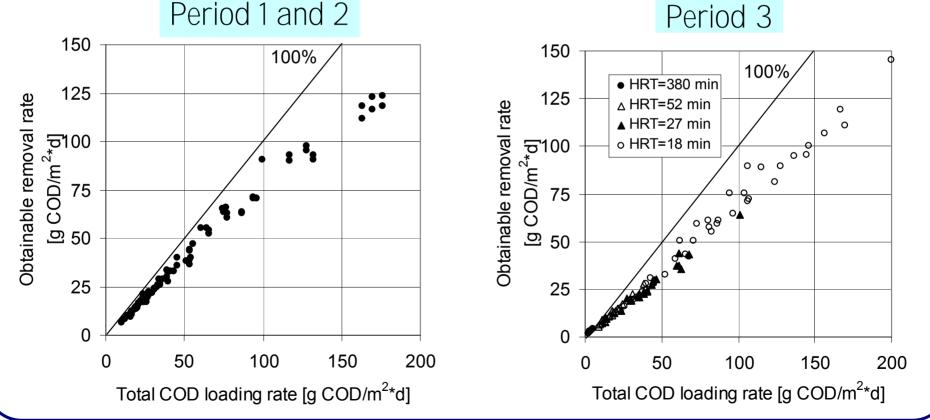


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"Obtainable" COD removal rate : (COD<sub>influent</sub>-SCOD<sub>effluent</sub>)\*Q/A

Period 3

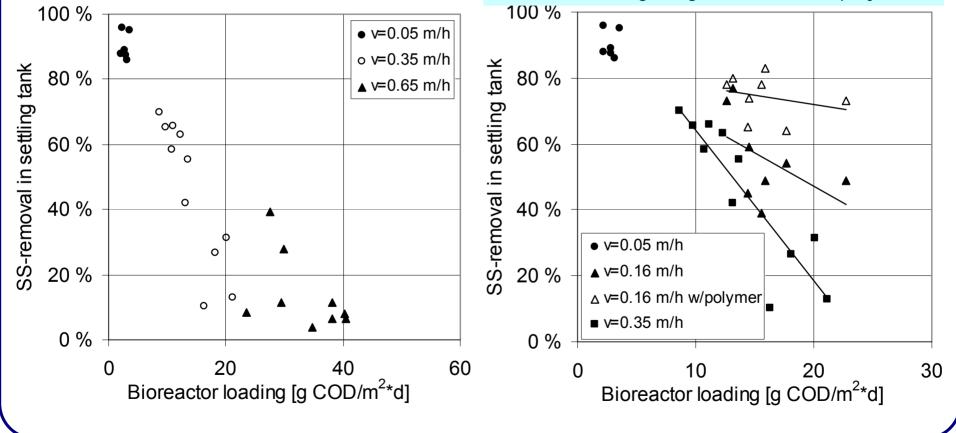


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## **RESULTS SETTLING EXPERIMENTS**

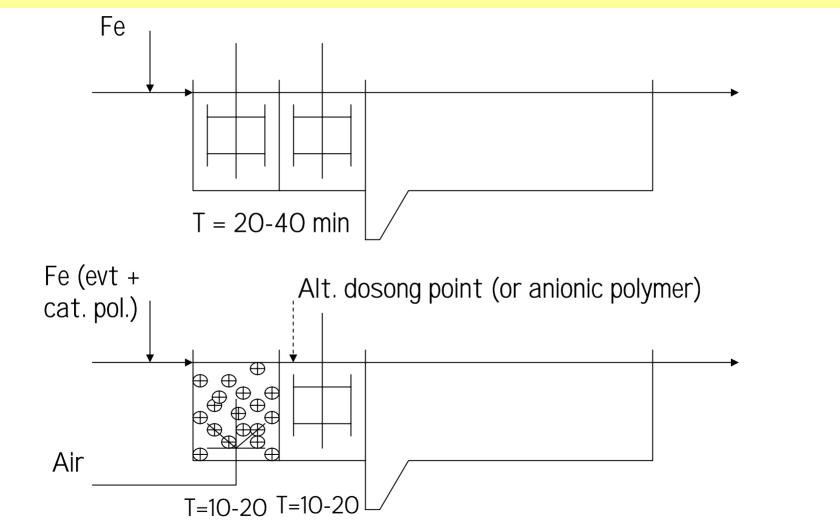
Influence of organic loading rate in bioreactor on settleability

Influence of polymer addition on settleability Medium charge, high MW cationic polymer



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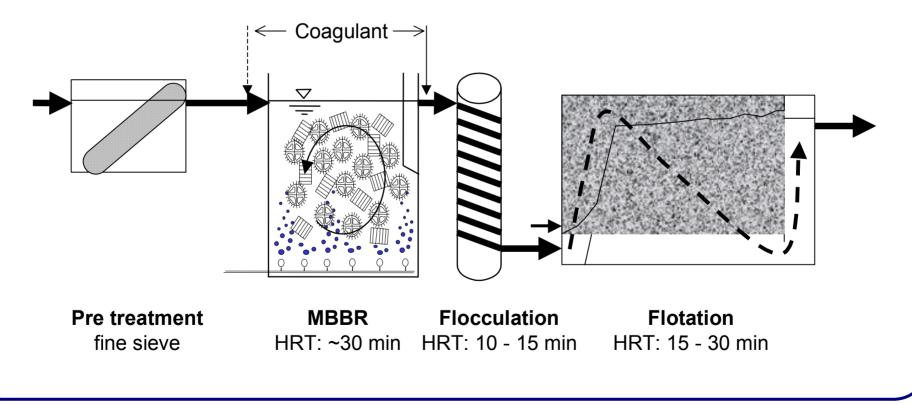
### RETROFITTING EXISTING PRIMARY PRECIPITATION PLANTS BY THE HIGH RATE MOVING BED PROCESS



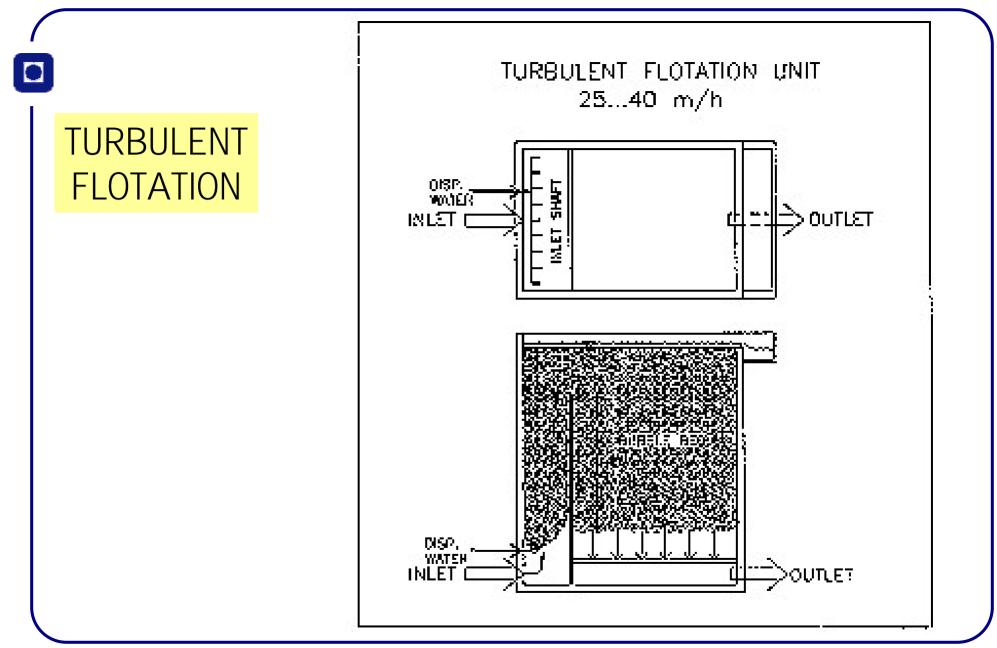
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# THE HIGH RATE PROCESS IN A NEW PLANT (total residence time < 1 time)



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# QUESTIONS TO BE ANSWERED

ON COAGULATION/FLOCCULATION

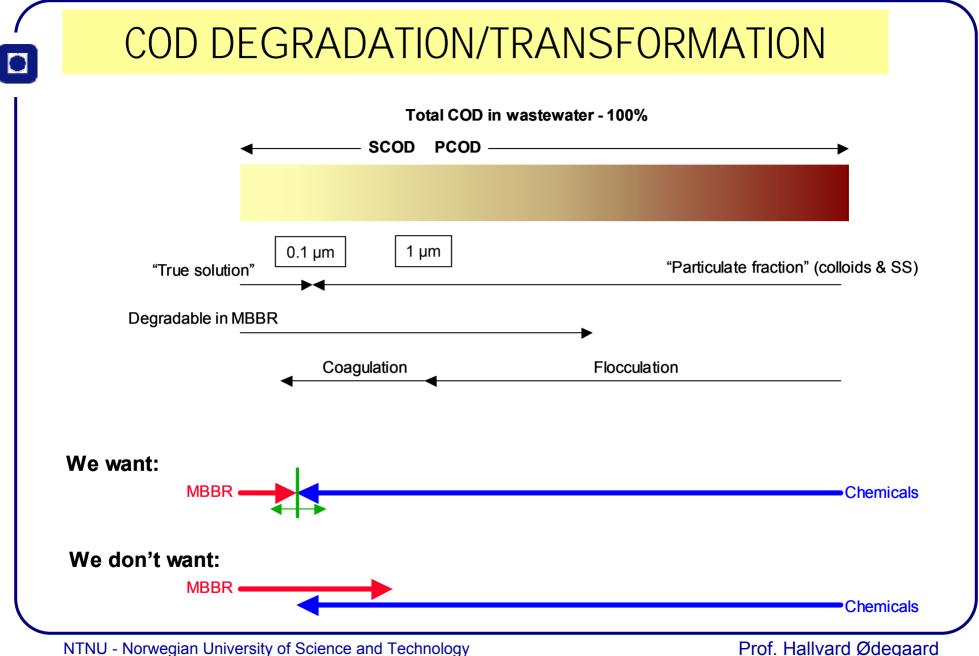
- Where to dose the coagulant?
- What coagulant?
- How much coagulant?

ON THE BIOREACTOR

- What happens to the soluble COD?
- What happens to the particulate COD?
- How to design the MBBR

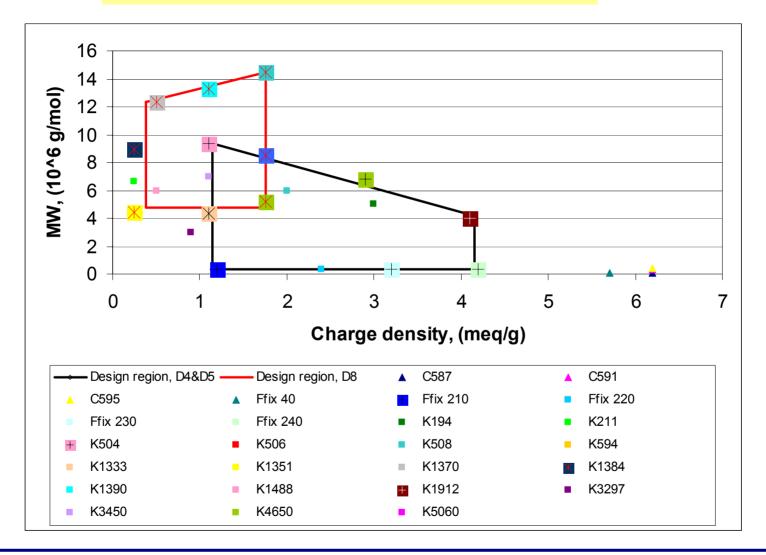
### ON THE FLOTATION REACTOR

- Performance of turbulent flotation
- Design of turbulent flotation



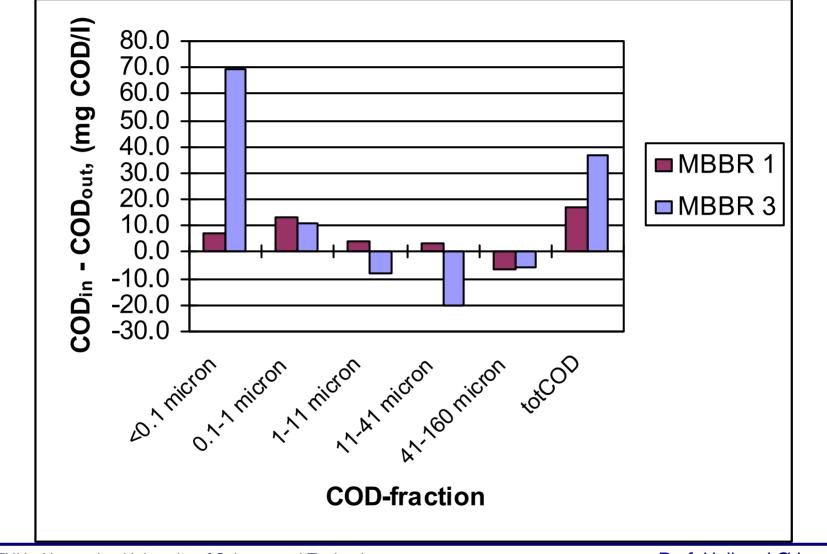
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### POLYMERS INVESTIGATED



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#### Net change in COD fractions through the MBBR Positive value: net removal, negative value: net production



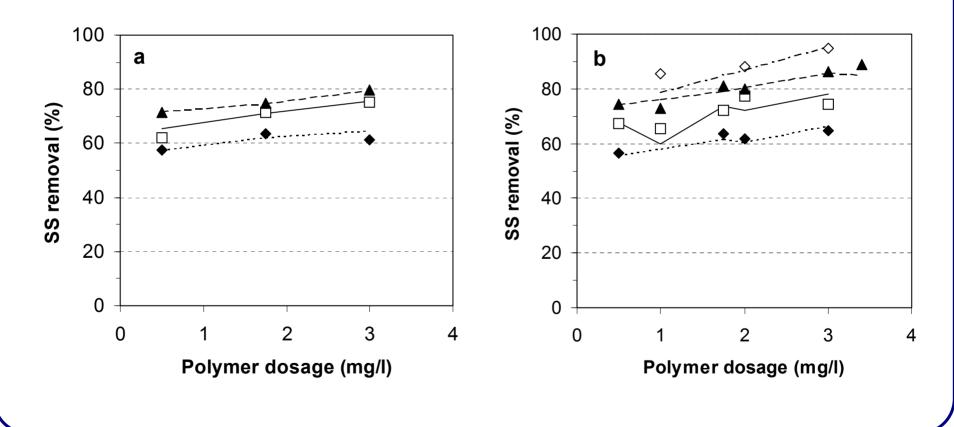
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#### EFFECT OF POLYMER AND IRON DOSAGE ON SS REMOVAL

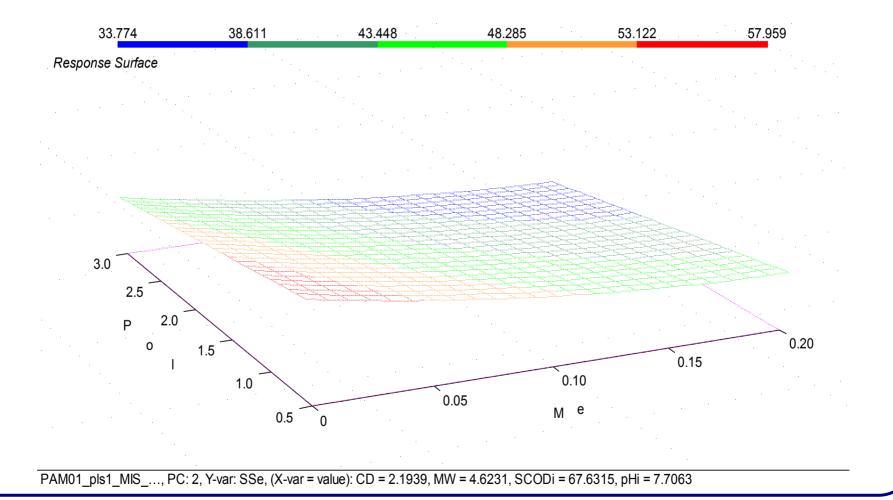
#### Polyamine

#### Polyacrylamide

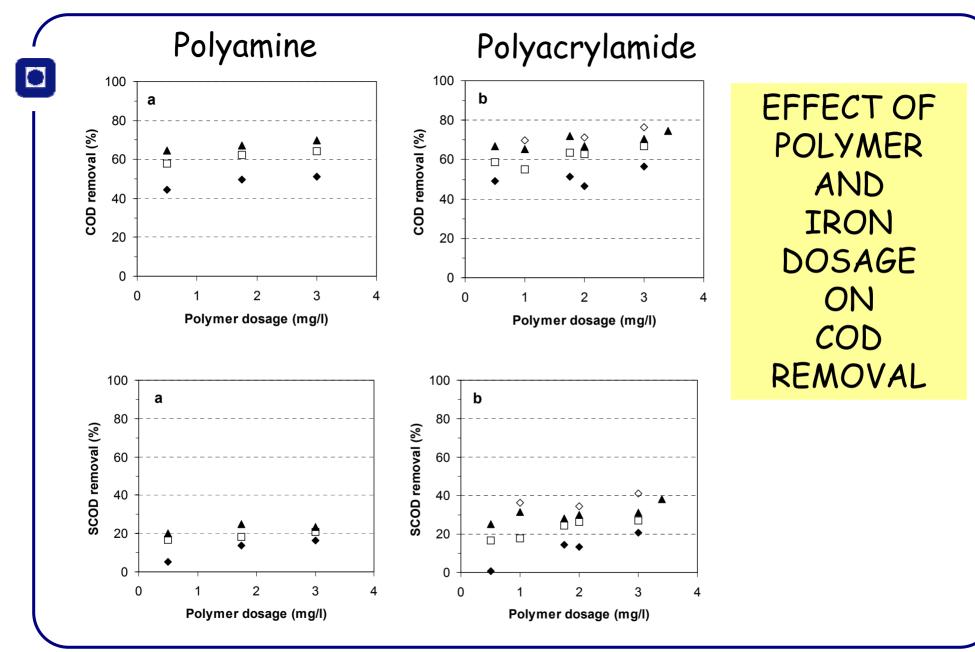


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#### **DETERMINATION OF OPTIMAL Fe/Polymer-DOSE**

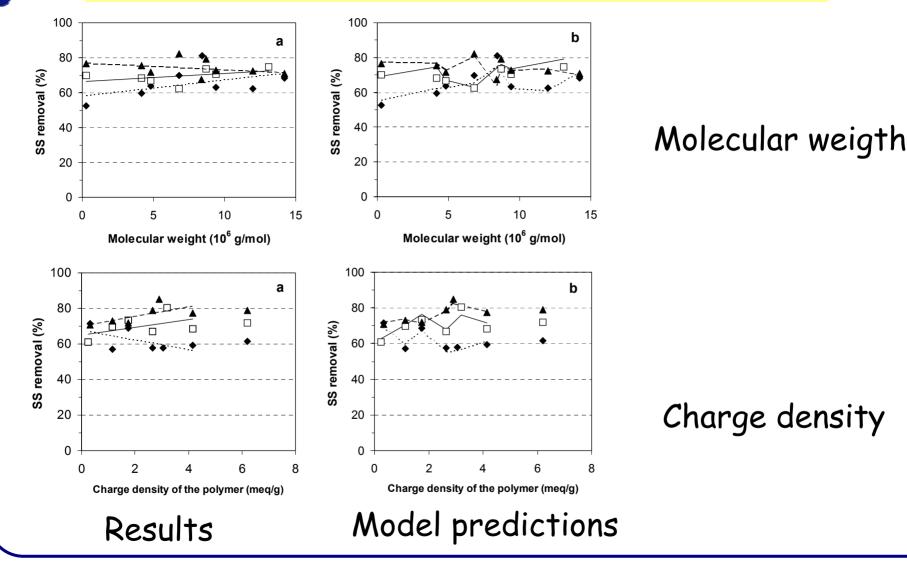


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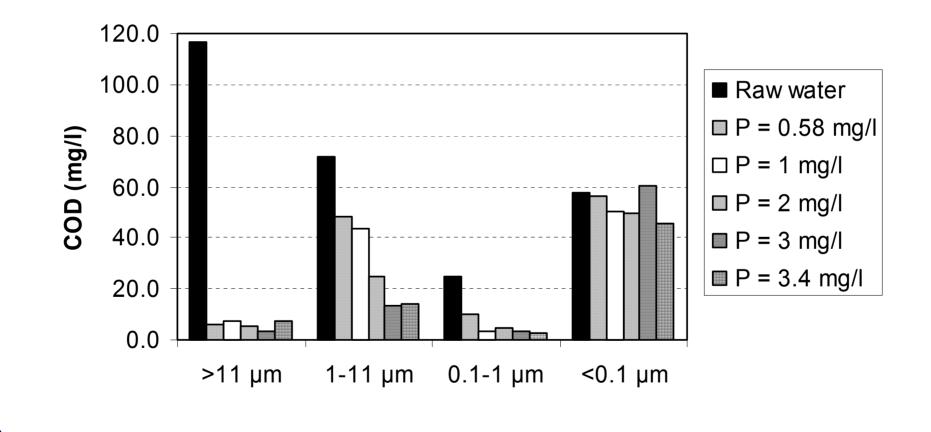
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# EFFECT OF POLYMER PROPERTIES



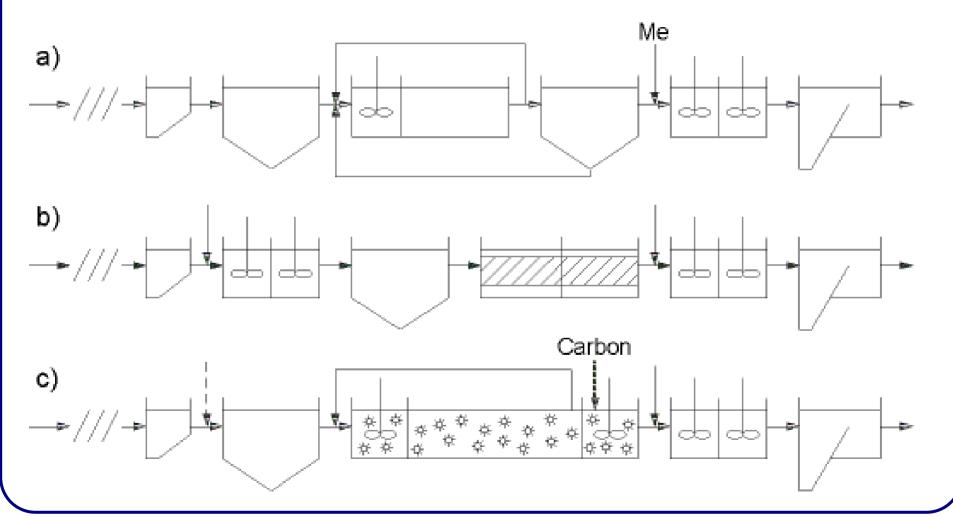
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#### PARTICULATE COD FRACTIONS IN RAW WATER AND FLOTATED WATER WITH DIFFERENT POLYDADMAC DOSAGES AND 0.2 MMOL FE/L IRON.



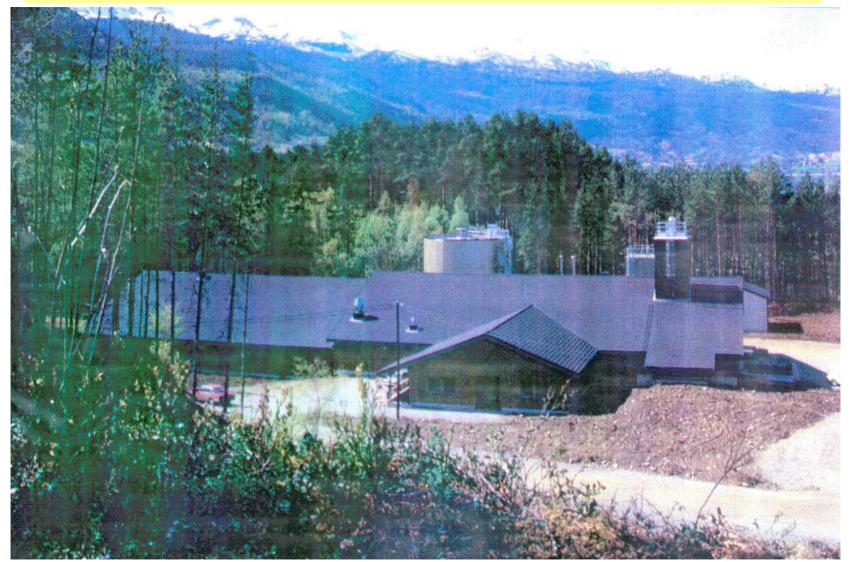
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### FLOTATION FOR FINAL SEPARATION

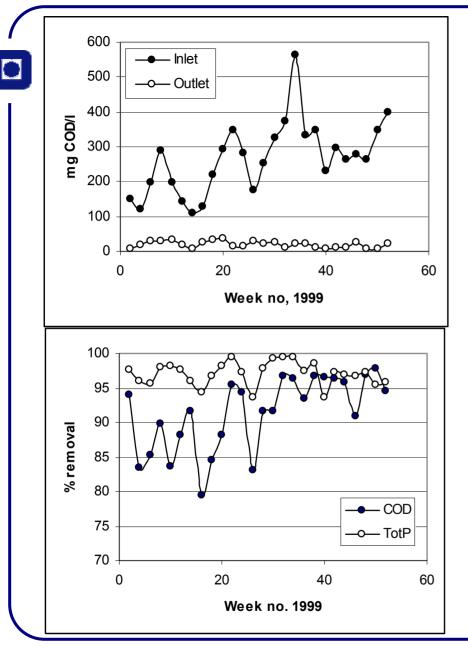


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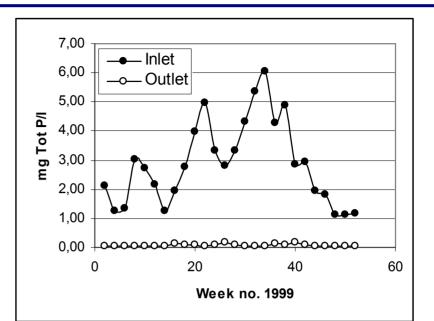
## VOSS WASTEWATER TREATMENT PLANT



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#### DATA VOSS TREATMENT PLANT

Overflow rate: 3 - 9 m/h

Effluent COD concentration : 19.6 + 8.7 mg/l

COD-removal efficiency : 91.4 + 5.2

Effluent Tot P concentration: 0.07 + 0.04 mg/l

Tot P removal efficiency : 97.1 <u>+</u>1.6

Secci-depth in effluent : 2.0 - 3.75 m

### GARDERMOEN WWTP



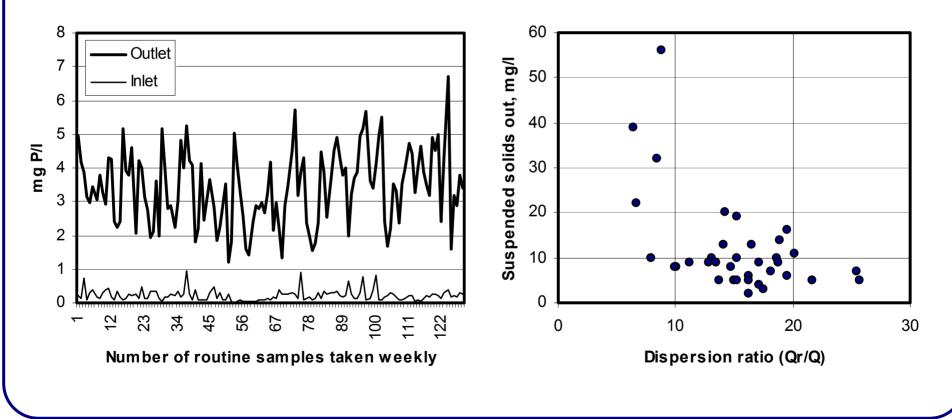
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### OPERATIONAL EXPERIENCES NORDRE FOLLO

Inlet and outlet tot P conc.





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### SUMMARY

- A very good particle removal and hence organic matter removal can be obtained by the use of pre-coagulation
- If advanced phosphate removal is not the objective, sludge production can be minimised by replacing part of the metal cation for coagulant by a organic polymer cation
- The addition of an anionic organic polymer as flocculant can improve the settleability of flocs dramatically with corresponding reduction in plant space requirement
- The combination of a high-rate moving bed process and coagulation/flotation can result in an extremely compact and efficient secondary treatment process